

Module 205

SCS Runoff Equation

**Engineering
Hydrology Training Series**

Module 205—SCS Runoff Equation

National Employee Development Center
Natural Resources Conservation Service
United States Department of Agriculture
July 1999

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Preface

This module consists of a study guide. It is intended as a self-paced package.

This study guide contains a step-by-step approach for using the SCS runoff equation. Examples are provided as a pattern for your use. The guide should serve as a reference to be used later, if needed, for review.

Proceed through this module at your own pace. Be sure you completely understand each section before moving on. If you have questions or need help, please request assistance from your supervisor. If your supervisor cannot clear up the problem, he or she will contact the state assigned resource person. The resource person is familiar with the material and should be able to answer any questions you may have.

Acknowledgment

The design and development of this training program is the result of a concentrated effort by practicing engineers in NRCS. The contributions of many technical and procedural reviews have helped to make this program one that will provide needed hydrology and hydraulic skills to NRCS employees.

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Module Description

Objectives



Upon completion of this module, the participant will be able to:

- Describe water balance.
- Derive the SCS Runoff Equation.
- Explain the application of the SCS Runoff Equation.
- Explain Runoff Curve Numbers.
- List limitations on the use of the SCS Runoff Equation.
- Perform at ASK Level 3 (perform with supervision).

Prerequisites

Modules 103—Runoff Concepts and 105—Runoff Computations

Length

Participant should take as long as necessary to complete this module. Training time for the module is about 1 one hour.

Who May Take Module

This module is intended for all NRCS personnel who use the runoff equation in their work.

Method of Completion

This module is self-paced, but the state should select a resource person to answer any questions that the participant's supervisor cannot handle.

Contents

This module discusses the concepts assumed in the rationale for the basic equation. Parameters are defined and explained. The runoff equation is developed step by step, and attention is given to correctness in use of the parameters as well as the algebra.

Module 205—SCS Runoff Equation

Introduction

The NRCS method of estimating direct runoff from rainfall was developed by SCS hydrologists during the early 1950's. The primary uses for such estimates are to establish safe limits for hydrologic design and to compare the effects of alternative conservation measures in a watershed. Runoff needs for NRCS work include volume estimates for "short" duration detention and peak discharge estimates.

Watersheds that NRCS are generally concerned with are small and un-gaged (no stream-gage records available), thus any runoff procedures should be appropriate for these conditions. Data that are generally available or that can be easily obtained for a watershed include:

- Watershed location
- Published rainfall-frequency data
- Land use and cover information
- Soils information

Considering the needs of NRCS and the data that were available at the time the model was developed, an event model with generalized runoff parameters was used. Streamflow data for small watersheds were available from the Agricultural Research Service and the U.S. Geological Survey. This information formed the data base for the runoff curve numbers that would be used in the SCS runoff equation.

Water Balance

A water balance accounts for all water that enters and leaves a system. For our needs, the system is a watershed and the balance is:

$$\text{Rainfall} = \text{Runoff} + \text{Losses}$$

Solving the water balance for runoff yields:

$$\text{Runoff} = \text{Rainfall} - \text{Losses}$$

Runoff

Runoff is classified by type as it flows through the watershed. The runoff types and their characteristics are:

- Channel runoff—Rainfall that falls on the watercourse. Generally this is a negligible quantity.
- Surface runoff—Generated when the rainfall rate exceeds the infiltration rate.
- Subsurface flow—The horizontal movement of infiltrated water in the soil. Subsurface flow may reappear as surface runoff shortly after rainfall (through seeps or springs). It can also be referred to as “quick return flow” or “interflow.”
- Direct runoff—A collective term that includes all the above runoff types.
- Baseflow—A fairly steady release of water from natural or manmade storage areas, such as lakes, swamps, or maybe underground aquifers. It is the flow that lingers on after the immediate effects of a runoff event have occurred.

Rainfall

Rainfall as used in the SCS runoff equation is considered an event amount. Data that were available for development of the procedure were not for events, but for calendar days or 24-hour periods. During times of thunderstorms, either of these amounts could include more than one actual event.

Losses

Losses include rainfall interception, soil infiltration, surface storage, and evaporation.

- *Interception*—Rainfall that does not reach the ground because it has contacted something (generally vegetation or buildings).
- *Infiltration*—Water that reaches the ground and enters the soil. Infiltrated water may become part of subsurface flow or the ground water table.
- *Surface storage*—Water that reaches the ground and is collected in depressions (low spots) along the flow path or in a closed basin. Water in surface storage must either infiltrate or evaporate.
- *Evaporation*—Water that goes back to the atmosphere. It is usually a negligible amount.

Water Balance Equation

Expressing the water balance as an equation:

$$Q = P - (I_a + F)$$

where:

$$Q = \text{direct runoff (ins)}$$

$$P = \text{rainfall (ins)}$$

$$I_a = \text{sum of all losses before the beginning of runoff (ins)}$$

and

$$F = \text{retention after runoff begins (ins)}$$

The water balance equation has four parameters. During development of the SCS runoff equation, Q and P were measured in the field. I_a could have been computed but a generalized relationship was developed from data. F was solved for and generalized as the variable S (which will be explained later).

For the application phase of the equation, Q is the quantity that is unknown. P can be obtained from either actual measurements or generalized rainfall-frequency analysis. I_a and F are functions of S and are fitted based on the development work. S was created to express I_a and F in terms of one equation fitting parameter and represents the hypothetical limit of storage. It is defined as the potential maximum retention after runoff begins. S lumps all variation in the runoff response because of land use, soils, soil moisture, or rainfall pattern, duration, or intensity, plus any other variation into one variable.

Because all of the losses are grouped together and not defined by amount and source, the model to be developed is called a “lumped system” model.

SCS Runoff Equation

To transform the water balance equation to what is called the SCS runoff equation, two assumptions were made:

Assumption # 1

The ratio of the percent water that has been retained to the maximum potential retention is the same as the ratio of the percent water that ran off to the maximum rainfall available for runoff. This assumption is expressed as:

$$F/S = Q/(P-I_a)$$

where F is the amount of rainfall retained (after runoff begins);
 S is the maximum potential retention (after runoff begins);
 Q is the amount of runoff, and
 $P-I_a$ is the maximum rainfall available for runoff.

At the limit where P is exceptionally large:

$$Q/(P-I_a) \ll 1, \text{ and}$$

$$F/S \ll 1.$$

When no runoff occurs, $P = I_a$, both F and Q are zero.
Therefore:

$$F/S \ll 0, \text{ and}$$

$$Q/(P-I_a) \ll 0$$

Because these ratios are equal in their extremes, they are assumed to exhibit similar characteristics throughout their range. To get the generalized rainfall-runoff relation, solve both the water balance and the assumption #1 ratios for F and equate, thus eliminating F.

Water balance	$F = (P - I_a) - Q$
Assumption #1	$F = QS/(P - I_a)$
Equate	$(P - I_a) - Q = QS/(P - I_a)$
Multiply by (P - I _a)	$(P - I_a)[(P - I_a) - Q] = QS$
Group Q on one side	$(P - I_a)^2 = Q(P - I_a + S)$
Solve for Q	$(P - I_a)^2 / (P - I_a + S) = Q$

Assumption #2

I_a can be expressed as a function of S. If I_a is expressed as a function of one of the other equation parameters, then the runoff equation is greatly simplified because only rainfall and one fitting parameter (S) are needed to solve for runoff. The relationship that is used by NRCS is:

$$I_a = 0.2 S.$$

Substituting for I_a in the generalized runoff equation produces

$$Q = (P - 0.2 S)^2 / (P - 0.2 S + S)$$

and collecting terms produces the SCS runoff equation:

$$Q = (P - 0.2 S)^2 / (P + 0.8 S)$$

During development, I_a was measured at sites that had both continuous rainfall and runoff records. With both of these known, a solution for S could be found from the water balance and assumption # 1. A log plot of S versus I_a for many such events produced the scatter diagram in figure 1. The mean value of the relation ($I_a = 0.2 S$) was selected. The data used to develop figure 1 were primarily from small agricultural watersheds and represent a wide range of storm conditions.

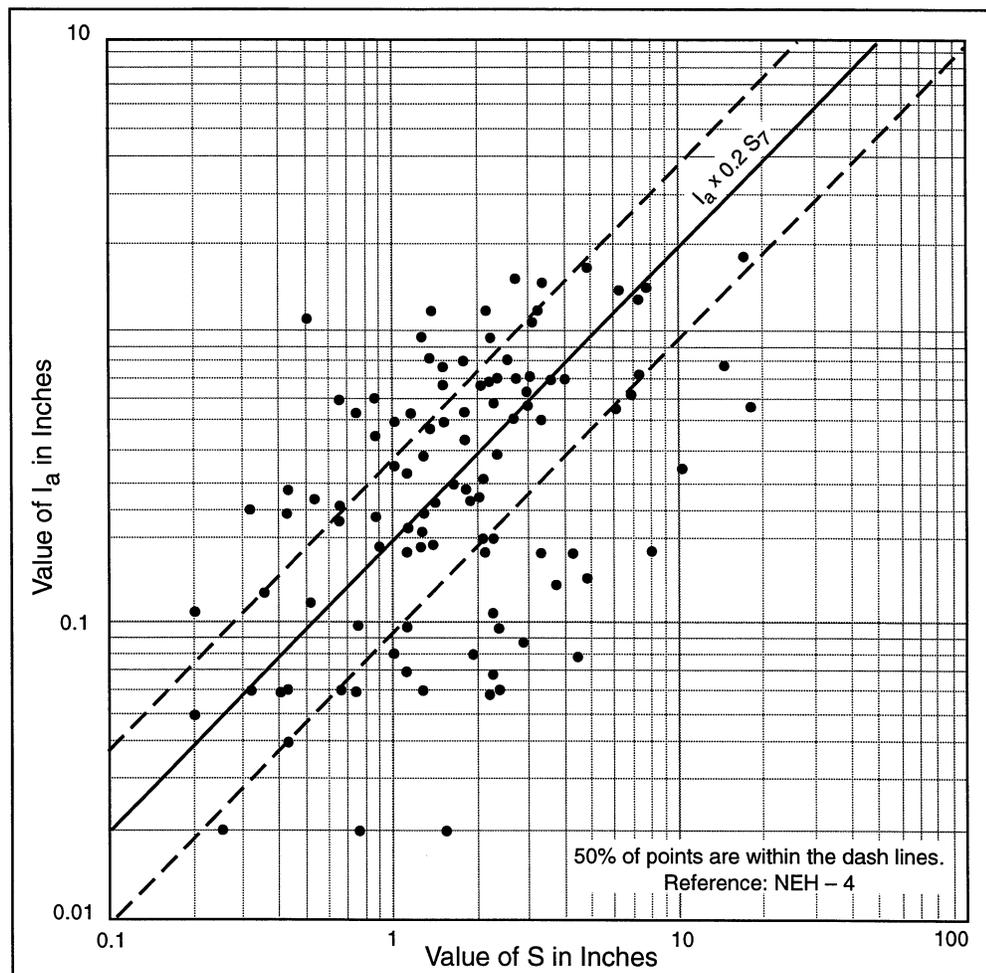


Figure 1. Relationship of I_a and S based on experimental watershed data.
Reference: NEH-4.

Runoff Curve Numbers (CN)

The potential maximum retention after runoff begins (S) has a range of values from 0 to infinity. S also requires the use of several decimal places to achieve any type of accuracy in its practical range (0 to 15). To use a more convenient value, a new parameter called curve number (CN), was established. The relationship between CN and S is an inverse relationship and the range of CN is limited by 0 and 100. Its practical range is 40 to 98. CN is also used only as an integer value.

The relationship between CN and S is:

$$CN = \frac{1000}{(S + 10)} \text{ or } S = \frac{1000}{CN} - 10$$

Table 1 is a solution of the above equations for the range of curve numbers 40 to 98.

Tables that give the CN to use in the runoff equation for various cover types and hydrologic soil groups are available. The cover types describe not only what is on the land, but in some cases its condition from a hydrologic standpoint (good, fair, or poor). The soils for the site are classified into one of four hydrologic soil groups, depending on the soils' ability to infiltrate water. The soil groups are called A, B, C, and D, which indicate the greatest infiltration capacity to the least, respectively. Cover/hydrologic soil group/CN data are given in table 9.1 in the National Engineering Handbook, Section 4 (Hydrology); Tables 2-3a through 2-3d in the Engineering Field Manual and Table 2-2a through 2-2d in Urban Hydrology for Small Watersheds (TR-55). The tables have been expanded over time and the most complete table is in TR-55. Table 2 is a sample.

Curve Number (CN)	Potential Maximum Retention (S)		Curve Number (CN)	Potential Maximum Retention (S)
40	15.000		70	4.286
41	14.390		71	4.085
42	13.810		72	3.889
43	13.256		73	3.699
44	12.727		74	3.514
45	12.222		75	3.333
46	11.739		76	3.158
47	11.277		77	2.987
48	10.833		78	2.821
49	10.408		79	2.658
50	10.000		80	2.500
51	9.608		81	2.346
52	9.231		82	2.195
53	8.868		83	2.048
54	8.519		84	1.905
55	8.182		85	1.765
56	7.857		86	1.628
57	7.544		87	1.494
58	7.241		88	1.364
59	6.949		89	1.236
60	6.667		90	1.111
61	6.393		91	0.989
62	6.129		92	0.870
63	5.873		93	0.753
64	5.625		94	0.628
65	5.385		95	0.526
66	5.152		96	0.417
67	4.925		97	0.309
68	4.706		98	0.204
69	4.493			
70	4.286			

Table 1. Curve numbers and equivalent potential maximum retention values.

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Cover description			Curve numbers for hydrologic soil group—			
Cover type	Treatment*	Hydrologic condition**	A	B	C	D
Fallow	Bare Soil	—	77	86	91	94
	Crop residue cover (CR)	Poor	76	85	90	93
		Good	74	83	88	90
Row crops	Straight row (SR)	Poor	72	81	88	91
		Good	67	78	85	89
	SR + CR	Poor	71	80	87	90
		Good	64	75	82	85
	Contoured (C)	Poor	70	79	84	88
		Good	65	75	82	86
	C + CR	Poor	69	78	83	87
		Good	64	74	81	85
	Contoured & terraced (C&T)	Poor	66	74	80	82
		Good	62	71	78	81
C&T + CR	Poor	65	73	79	81	
	Good	61	70	77	80	
Small grain	SR	Poor	65	76	84	88
		Good	63	75	83	87
	SR + CR	Poor	64	75	83	86
		Good	60	72	80	84
	C	Poor	63	74	82	85
		Good	61	73	81	84
	C + CR	Poor	62	73	81	84
		Good	60	72	80	83
	C&T	Poor	61	72	79	82
		Good	59	70	78	81
	C&T + CR	Poor	60	71	78	81
		Good	58	69	77	80
Close-seeded or broadcast legumes or rotation meadow	SR	Poor	66	77	85	89
		Good	58	72	81	85
	C	Poor	64	75	83	85
		Good	55	69	78	83
	C&T	Poor	63	73	80	83
		Good	51	67	76	80

Table 2. Sample curve numbers. Reference: TR-55

Average runoff condition, and $I_a=0.2S$.

*Crop residue cover applies only if residue is on at least 5% of the surface throughout the year.

**Hydrologic condition is based on a combination of factors that affect infiltration and runoff, including (a) density and canopy of vegetative areas, (b) amount of year-round cover, (c) amount of grass or close-seeded legumes in rotations, (d) percent of residue cover on the land surface (good $\leq 20\%$), and (e) degree of surface roughness.

Poor: Factors impair infiltration and tend to increase runoff.

Good: Factors encourage average and better than average infiltration and tend to decrease runoff.

The CN's published in these tables are for an average runoff condition. While the exact conditions that are considered average are not defined, some items that can cause runoff conditions to vary are:

- Rainfall amount,
- Rainfall pattern,
- Maximum rainfall intensity,
- Rainfall duration,
- Antecedent soil moisture, and
- Air temperature.

When the curve number tables were originally developed, sample watershed P and Q data pairs were plotted to produce a scatter of points. The CN for each event was determined by “reverse engineering” the runoff equation to solve for S and then converting S to CN. A median curve number was determined and plotted along with the sample data. This median CN represents the average runoff condition. The range of the remaining CN's were noted and later smoothed mathematically. This range is considered to define the limiting values for a specified average runoff condition CN. Table 3 lists the limiting CN values.

Theoretically, any value within the range could be expected to occur in a watershed for some event, some time. Thus if rainfall and runoff data are available for an event, an event CN could be computed. This event CN should fall within the range listed for the appropriate average condition CN. Also, any CN within the range could be used for watershed analysis and design. Any variation from the average condition CN should be documented. In most situations the average condition value is adequate.

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Average Condition CN	Lower Limit CN	Upper Limit CN		Average Condition CN	Lower Limit CN	Upper Limit CN
40	22	60		70	51	85
41	23	61		71	52	86
42	24	62		72	53	86
43	25	63		73	54	87
44	25	64		74	55	88
45	26	65		75	57	88
46	27	66		76	58	89
47	28	67		77	59	89
48	29	68		78	60	90
49	30	69		79	62	91
50	31	70		80	63	91
51	31	70		81	64	92
52	32	71		82	66	92
53	33	72		83	67	93
54	34	73		84	68	93
55	35	74		85	70	94
56	36	75		86	72	94
57	37	75		87	73	95
58	38	76		88	75	95
59	39	77		89	76	96
60	40	78		90	78	96
61	41	78		91	80	97
62	42	79		92	81	97
63	43	80		93	83	98
64	44	81		94	85	98
65	45	82		95	87	98
66	46	82		96	89	99
67	47	83		97	91	99
68	48	84		98	94	99
69	50	84				
70	51	85				

Table 3. Curve number ranges for average condition curve numbers.

Application Of The SCS Runoff Equation

Before the SCS runoff equation can be used, the land area, cover description, hydrologic soil group, and rainfall frequency must be determined.

Land area and cover description

Field observation, aerial photographs or a combination of these topographic maps, can be used to determine the land area and cover description.

Hydrologic soil group

The published soil survey for the site can be used to determine appropriate soil names. These names, in turn, are referred to on the applicable table where the hydrologic soil groups are found. Tables, such as table 7.1 in NEH Section 4, Exhibit 2-1 in the EFM; or Exhibit A-1 in TR-55 should be used. Table 4 shows a sample soil name/hydrologic soil group table.

Curve number

Using the cover description and the hydrologic soil group, a curve number can be determined from the CN tables. If more than one cover or hydrologic soil group are present, a weighted CN must be determined to represent the entire area. CN values are weighted by the applicable area to which they pertain.

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Lamkin	B	Lariat	B	Lavina	D	Leetonia	C	Lew	B
Lamo	C	Larim	B	Lavon	C	Leevan	C	Lewbeach	C
Lamdille	B	Larimer	B	Lawai	B	Lefor	B	Lewdlac	D
Lamondi	B	Larioscamp	D	Lawen	B	Legall	B	Lewis	D
Lamoni	C	Larkin	B	Lawet	B/D	Legault	D	Lewisberry	B
Lamont	B	Larkson	C	Lawet,	B	Leggett	C	Lewisburg	C
Lamonta	D	Larmine	D	Saline-Alkali		Legler	B	Lewiston	C
Lamoose	D	Laroque	B	Lawler	B	Legore	B	Lewisville	B
Lamotte	B	Larose	D	Lawndale	B	Lehew	C	Lewkalb	C
Lamoure	C	Larrupin	B	Lawnwood	B/D	Lehigh	C	Lex	B
Lampasas	D	Larry	D	Lawnwood,	D	Lehmans	D	Lexington	B
Lampmier	B	Larry, Drained	C	Depressional		Lehr	B	Lexton	B
Lampshire	D	Larson	D	Lawrence	C	Leicester	C	Leyba	B
Lamson	B/D	Larton	A	Lawrenceville	C	Leidl	C	Leyden	C
Lanark	B	Larue	A	Lawshe	D	Leighcan	B	Libbings	D
Lancaster	B	Larush	B	Lawson	C	Leilehua	B	Libeg	B
Lance	B	Larvie	D	Lawther	D	Leisy	B	Liberal	D
Land	C	Las	C	Lawton	C	Lela	D	Libory	A
Land, Drained	B	Las Animas	C	Lawyer	B	Leland	D	Library	D
Landavaso	B	Las Flores	D	Lax	C	Lemha	A	Ljbase	C
Landco	C	Las Lucas	B	Laxal	B	Lembos	C	Licha	B
Lander	C	Las Posas	C	Laxton	C	Lemco	C	Lick	B
Landes	B	Las Vegas	D	Laycock	B	Lemert	D	Lickdale	D
Landlow	C	Lasa	A	Layoint	C	Lemeta	D	Licking	C
Landman	B	Lasalle	D	Layton	A	Leming	C	Lickskillet	D
Landsend	C	Lasauses	D	Layview	D	Lemitar	D	Lidan	C
Lane	C	Lasco	B	Lazak	D	Lemm	B	Liddell	B/D
Lanesboro	C	Lasil	D	Lazear	D	Lemolo	D	Liddieville	B
Lanexa	D	Laska	B	Le Bar	B	Lemond	B/D	Lidy	B
Laney	B	Lassel	C	Le Sueur	B	Lemonex	C	Lieberman	B
Lang	C	Lassen	D	Lea	C	Lemoore	C	Lien	D
Langford	B	Lassiter	B	Leader	B	Lempira	B	Liesnoi	D
Langhei	C	Lastance	B	Leadore	B	Len	C	Ligget	B
Langlade	B	Latah	D	Leadpoint	C	Lena	A/D	Lightning	D
Langlois	D	Latah, High	C	Leadvale	C	Lena, Flooded	D	Lignum	C
Langola	B	Rainfall, Drained		Leadville	B	Lenapah	D	Ligon	D
Langrell	B	Latah, Drained	C	Leaf	D	Lenawee	B/D	Ligurta	B
Langspring	B	Latahco	C	Leafriver	A/D	Lenawee, Pondered	D	Lihen	A
Langston	B	Latahco, Wet	D	Leafu	C	Lenberg	C	Lihue	B
Langtry	D	Latanier	D	Leagueville	B/D	Lennep	C	Likes	A
Lanier	A	Latch	A	Leaksville	D	Lenoir	D	Lilah	A
Laniger	B	Latene	B	Leal	B	Lenz	B	Lilbert	B
Laniger, Gravelly	C	Lates	C	Lealandic	D	Lenz, Stony	C	Lilbourn	B
Lankbush	D	Latex	C	Leanna	C	Lenz, Very Stony	C	Lillings	B
Lankin	C	Latham	D	Leanto	D	Lenzburg	B	Lillington	B
Lanktree	C	Lather	D	Leaps	C	Leo	A	Lillylands	C
Landak	B	Lathrop	B	Leathan	C	Leola	B	Lilten	C
Lanona	B	Latigo	B	Leatherman	D	Leon	B/D	Lily	B
Lansdale	B	Latina	D	Leavenworth	C	Leonard	D	Lim	C
Lansdowne	C	Latium	D	Leavers	B	Leonardo	B	Lima	B
Lansing	B	Laton	D	Leavitt	B	Leonardtown	D	Limber	B
Lantern	B	Latonia	B	Leavittville	B	Leoni	B	Limekiln	D
Lantis	B	Latouche	D	Lebam	B	Leguieu	D	Limerick	C
Lanton	D	Latour	B	Lebanon	C	Lerdal	C	Limeridge	D
Lanton, Low	C	Latourell	B	Lebeau	D	Lerdo	C	Limking	B
Precipitation		Lattas	D	Lebec	B	Leroy	B	Limon	C
Lantonia	B	Latty	D	Lebo	B	Lerrow	C	Limon, Wet	D
Lantry	B	Lauderdale	D	Lebsack	C	Leshara	B	Limones	B
Lantz	D	Lauderhill	B/D	Leck Kill	B	Lesho	C	Limpia	C
Lanver	C	Laufer	D	Lecrag	D	Leslie	D	Linco	B
Lanyon	C/D	Laugenour, Loamy	C	Ledford	B	Leson	D	Lincoln	A
Lap	D	Substratum		Ledgfork	A	Lespate	C	Lindaas	C/D
Laparita	C	Laugenour, Silty	B	Ledmound	D	Lester	B	Lindale	C
Lapdun	B	Substratum		Ledow	B	Leswill	B	Lindell	C
Laped	D	Laugenour, Drained	B	Ledru	C	Leta	C	Linden	B
Lapeer	B	Laughlin	C	Ledub	B	Letcher	D	Linder	B
Lapham	A	Laumaia	B	Ledwith	B/D	Letha	C	Lindley	C
Lapine	A	Laurel	D	Lee	D	Letment	D	Lindrith	B
Laplatta	C	Laurelwood	B	Leebench	D	Letney	A	Lindside	C
Lapon	D	Lauren	B	Leeds	C	Leton	D	Lindstrom	B
Laporte	D	Laurentzen	D	Leefield	C	Letort	B	Lindy	C
Laposa	C	Lavacreek	B	Leeko	C	Letri	B/D	Line	B
Lapwai	B	Lavallee	B	Leeko, Warm	B	Lettia	B	Lineville	C
Larand	B	Lavate	B	Leelanau	A	Levasy	C	Linganore	B
Larchmont	B	Laveaga	C	Leemont	D	Levelton	D	Linhart	A
Lardell	C	Laveen	B	Leeper	D	Levelton, Drained	C	Lining	C
Laredo	B	Laventana	B	Leeray	D	Leverett	C	Linker	B
Lares	C	Laverkin	C	Leesburg	B	Leviathan	B	Linkup	D
Largo	B	Lavic	B	Leesville	B	Levy	D	Linkville	B

Table 4. Sample hydrologic soil groups for United States soils. Note: Two hydrologic soil groups such as B/C indicates the drained/undrained situation. Modifiers shown, e.g., bedrock saturation. Refer to a specific soil series phase found in soil map legend.

Rainfall frequency

The rainfall frequency will be determined by the purpose for the runoff estimate. This may be a design specification for a proposed engineering practice or a policy specification for an evaluation. Once the frequency is known, the 24-hour rainfall can be determined in many sites from a predetermined list of rainfall frequency values. It can also be determined by reading a value from a National Weather Service Map. Figure 2 lists the sources of rainfall frequency data for different geographic areas.

Runoff determination

Runoff can be determined from one of several forms of the runoff equation. The equation itself can be used. Graphs of P and Q by CN are available, and, if you like tables, a $P - Q - CN$ table is available. Needless to say, the equation is the most accurate. Some interpolation may be required (graphical or mathematical) for the other methods. The graphs are in Figure 10.1 in NEH, Section 4; Figure 2-1 in TR-55. Figure 3 shows the TR-55 graph. Exhibit 2-7 in the EFM; Table 2-1 in TR-55; and TR-16 are look-up tables. The TR-16 table is the most complete. Table 5 is a sample of a TR-16.

East of 105th meridian

Hershfield, D.M. 1961. Rainfall frequency atlas of the United States for durations from 30 minutes to 24 hours and return periods from 1 to 100 years. U.S. Dep. Commerce, Weather Bureau Tech. Pap. No. 40. Washington, DC. 115p.

West of 105th meridian

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Alaska

Miller, John F., 1963. Probable maximum precipitation and rainfall-frequency data for Alaska for areas to 400 square miles durations to 24 hours and return periods from 1 to 100 years. U.S. Dep. Commerce, Weather Bur. Tech. Pap. No. 47. Washington, DC. 69 p.

Hawaii

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Puerto Rico and Virgin Islands

Weather Bureau. 1961. Generalized estimates of probable maximum precipitation and rainfall-frequency data for Puerto Rico and Virgin Islands for areas to 400 acres, durations to 24 hours, and return periods from 1 to 100 years. U.S. Dep. Commerce, Weather Bur. Tech. Pap. No. 42. Washington, DC. 94 p.

Figure 2. Sources of 24 hr. rainfall data.

Runoff for Inches of Rainfall (Curve No. 75)										
Rainfall (Inches)	Rainfall (Tenths)									
	0.0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	.09
0								0.00	0.01	0.02
1	.003	0.05	0.07	0.10	0.13	0.17	0.20	.024	0.29	0.33
2	0.38	0.43	0.48	0.54	0.59	0.65	0.71	0.77	0.83	0.90
3	0.96	1.03	1.10	1.16	1.23	1.30	1.37	1.45	1.52	1.59
4	1.67	1.74	1.82	1.90	1.97	2.05	2.13	2.21	2.29	2.37
5	2.45	2.53	2.61	2.70	2.78	2.86	2.95	3.03	3.11	3.20
6	3.28	3.37	3.46	3.54	3.63	3.71	3.80	3.89	3.98	4.06
7	4.15	4.24	4.33	4.42	4.51	4.59	4.68	4.77	4.86	4.95
8	5.05	5.13	5.22	5.32	5.41	5.50	5.59	5.68	5.77	5.86
9	5.95	6.05	6.14	6.23	6.32	6.42	6.51	6.60	6.69	6.79
10	6.88	6.97	7.07	7.16	7.25	7.35	7.44	7.53	7.63	7.72
11	7.82	7.91	8.00	8.10	8.19	8.29	8.38	8.48	8.57	8.67
12	8.76	8.86	8.95	9.05	9.14	9.24	9.33	9.43	9.52	9.62
13	9.71	9.81	9.90	10.00	10.09	10.19	10.29	10.38	10.48	10.57
14	10.67	10.77	10.86	10.96	11.05	11.15	11.25	11.34	11.44	11.54
15	11.63	11.73	11.82	11.92	12.02	12.11	12.21	12.31	12.40	12.50
16	12.60	12.69	12.79	12.89	12.99	13.08	13.18	13.28	13.37	13.47
17	13.57	13.67	13.76	13.86	13.96	14.05	14.15	14.25	14.35	14.44
18	14.54	14.64	14.74	14.83	14.93	15.03	15.13	15.22	15.32	15.42
19	15.52	15.61	15.71	15.81	15.91	16.00	16.10	16.20	16.30	16.40
20	16.49	16.59	16.69	16.79	16.88	16.98	17.03	17.18	17.28	17.37

Table 5. Solution of runoff equation. Reference: TR-16.

Application Limitations

The runoff equation is a simple and easy-to-use model that requires only a limited amount of data. The simplifying assumptions mentioned above, though, put constraints on the model that reduce its flexibility or utility, or both. Two important limitations are:

Time independence

For a constant CN, a given amount of rainfall produces a set amount of runoff regardless of the rate of rain fall or how fast the soil can infiltrate water. The lumped system does not identify what part of the loss is infiltration.

Fixed initial abstraction

For a constant CN, the soil moisture or soil properties are not used. While these may be factors in initially selecting the CN value, they are not used in the runoff computations. As an example, a selected CN could occur for soils in two different hydrologic soil groups (A and C) by varying land cover. A different runoff pattern should be expected because of the varying infiltration rates, but the amount of rain required before runoff begins will be the same for both. Thus the accuracy of runoff estimates is reduced for small amounts (0.5 in) of runoff.

Activity 1



Before continuing, complete Activity 1. After completing the activity, review the solution near the back of this module to check your answer. When you have finished, return to the Study Guide text.

Activity 1



1. Name the three major components in a water balance.

a. _____

b. _____

c. _____

2. Briefly state the two assumptions made to create the SCS runoff equation from a water balance.

3. Why were curve numbers used in place of the potential maximum retention after runoff begins?

4. List the types of data required to apply the SCS runoff equation.

a. _____

b. _____

c. _____

d. _____

e. _____

f. _____

5. What are the two major limitations on the runoff equation?

a. _____

b. _____

Summary

The SCS runoff equation is a lumped system parameter model. The CN's that are used in its application are derived from actual rainfall and runoff data for small watersheds. The model requires limited data (land cover, hydrologic soil group, and rainfall amount) and is especially applicable to ungaged areas that are typical in NRCS work. The runoff solution is simple to apply with the three forms of the equation available (mathematic, graph, and table). Two limitations of the equation are that it is time independent and accuracy is reduced for low runoff amounts because of the $I_a = 0.2 S$ assumption.

By now, you should be able to explain the derivation of and use the SCS Runoff Equation. If you feel you do not understand a portion, you should review that section. In addition, you should retain the Study Guide as a reference until you are satisfied that you can use the runoff equations.

When you are satisfied that you have completed this module, complete the Certification of Completion and send it, through channels, to your state training officer.

Activity Solutions

Activity 1

1. Name the three major components in a water balance.

a. *Rainfall*

b. *Runoff*

c. *Losses*

2. Briefly state the two assumptions made to create the SCS runoff equation from a water balance.

Water retention and runoff occur in the same proportion.

$$I_a = 0.2 S$$

3. Why were curve numbers used in place of the potential maximum retention after runoff begins?

S has a wide range of values but a narrow practical range, thus several decimal places are required. Curve numbers have a wider practical range than S, so that 2-digit integer values can be used.

4. List the types of data required to apply the SCS runoff equation.
 - a. *Land cover*
 - b. *Soils and hydrologic soil group*
 - c. *Drainage area for each land cover and hydrologic soil group*
 - d. *Curve number (derived from the above data)*
 - e. *Rainfall frequency*
 - f. *24 hour rainfall*

5. What are the two major limitations on the runoff equation?
 - a. *Time independence*
 - b. *Ia is fixed by CN or S*

Hydrology Training Series
Module 205—NRCS Runoff Equation

Certificate of Completion

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Module 205—NRCS Runoff Equation
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Signed _____ Supervisor/Trainer
_____ Participant

Completion of Hydrology Training Series
Module 205—NRCS Runoff Equation

is acknowledged and documented in the above-named employee's record.

Signed _____ Date _____
Training Officer

